

July 9, 2021

Ms. Melissa Williams
Larson Camouflage
1501 S. Euclid Avenue
Tucson, AZ 85713

Subject: Monopole Fall Zone Design
Site: VB US-NC-7009
3529 Edgefield Rd.
Greensboro, NC 27409

Ms. Williams,

The 125' AGL, 3 carrier, steel mono-pole structure proposed for the subject site is designed for a "fall zone" radius limited to 46'. This design characteristic assures that, in the highly unlikely event of a pole failure, the failure will occur at an elevation that is 46' below the top of pole.

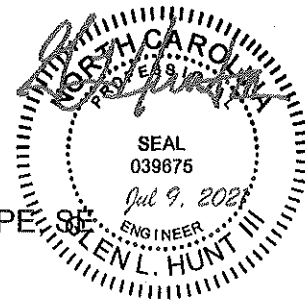
The pole design is prepared such that the top pole section has a design capacity usage of 98.6% at the maximum design load condition. The remaining pole sections, base plate, and anchor bolts are designed at 94% or less of available capacity. This design process forces an unlikely "failure" to occur in the top pole section before any other pole sections. Furthermore, failure and separation of the upper pole section will relieve load on the remaining tower sections ensuring survival.

Be advised that pole failure is an unlikely event and the pole design provided for this site will satisfy the requested fall radius specification.

If there are any questions regarding this issue, please feel free to give me a call.

Sincerely,

Glen L, Hunt III, MS, PE, SE
Principal Engineer





SITE #: US-NC-7009
 SITE NAME: MACAFFREY
 125'-0" MONOPINE

SHEET INDEX	
PF1	PRODUCT INFORMATION & NOTES
PF2	POLE ELEVATION
PF2.1	PIER FOUNDATION
PF3	DETAILS



ISE JOB #: 17030

PROJECT INFORMATION

DATE: June 17, 2021
 ISE JOB NO. 17030 By: MG
 CUSTOMER: LARSON-VALMONT
 PRODUCT: 125'-0" MONOPINE
 SITE ID: MACAFFREY US-NC-7009
 LOCATION: 3529 EDGEFIELD RD, GREENSBORO, NC 27409
 LATITUDE: 36° 08' 41.31" N
 LONGITUDE: 79° 57' 33.48" W

DESIGN CRITERION:

2018 IBC, 110 MPH BASIC WIND SPEED, TIA 222-H
 EXP C, TOPO CLASS I, TOWER CLASS II, ELEVATION 930 ft

POLE SPECIFICATIONS

Section Shape 18-Sided Tapered
 Pipe Taper 0.265IN/FT
 Pole Material ASTM A572-GR65
 Base Plate ASTM A572 GR 60
 Anchor Bolts 2.25"Ø x 84", ASTM A615-GR75

Pole Section	Length (ft.)	Weight (Kips)	Tks. (in.)	Lap Splice (in.)	Diameter	
					Top (in.)	Bot (in.)
L1	46.00	3.707	0.250	54.00	24.000	36.190
L2	49.00	9.389	0.438	70.92	34.498	47.483
L3	39.41	10.591	0.500		45.041	55.485

Base Plate 2.03 3.00 69.00"Ø w/ 41.50"Ø ID

TABLE INDICATES RAW STEEL WEIGHTS. FINAL GALVANIZED WEIGHTS SHALL BE APPROXIMATELY 22% GREATER.

EARTHQUAKE DESIGN DATA

IMPORTANCE FACTOR (I): I
 OCCUPANCY CATEGORY: II

S_v = 0.148 S_{ws} = 0.157
 S_i = 0.066 S_{is} = 0.106

SEISMIC DESIGN CATEGORY: B

SITE CLASS: D
 SEISMIC RESPONSE COEFFICIENT: 0.105
 DESIGN BASE SHEAR: 66.292 KIPS (WIND)
 RESPONSE MODIFICATION FACTOR (R): 1.50

**ANALYSIS PROCEDURE USED EQUIVALENT LATERAL FORCE PROCEDURE
 DESIGN LOADS (Unfactored Base Wind Reactions)**

Moment = 6321.247 Ft-Kips
 Shear = 66.292 Kips
 Axial = 44.323 Kips

DEFLECTIONS

Elev. (ft.)	60 MPH Wind		110 MPH Wind	
	Lateral (in.)	Sway (")	Lateral (in.)	Sway (")
Top	24.738	1.701	93.090	6.407

APPURTENANCES

Elevation (ft.)	(Qty)	Description
125'	(1)	4' Lightning Rod
70' - 125'	(138)	Assorted 4', 6', 8', 10' & 12' Pine Branches
120'	(1)	Carrier Provided Loading
109'	(1)	Carrier Provided Loading
99'	(1)	Carrier Provided Loading

GENERAL NOTES:

- ALL STEEL SHALL MEET THE REQUIREMENTS OF THE "STANDARD SPECIFICATIONS FOR STRUCTURAL STEEL" ASTM A36, UNLESS OTHERWISE NOTED ON THE STRUCTURAL PLANS OR BELOW.
- ALL ROUND STEEL PIPE SHALL MEET THE REQUIREMENTS OF API-5LX GR. 42 (42 KSI YIELD POINT MATERIAL).
- ALL TUBE STEEL (SQUARE OR RECTANGULAR) SHALL MEET THE REQUIREMENTS OF ASTM A500 GRADE B (46 KSI YIELD POINT MATERIAL).
- ALL POLYGON FORMED STEEL SHAFTS SHALL MEET THE REQUIREMENTS OF ASTM A572 GRADE 65 (65 KSI YIELD POINT MATERIAL).
- ALL WELDED CONNECTIONS SHALL CONFORM TO THE LATEST VERSION OF THE AMERICAN WELDING SOCIETY AWS D1.1 CODE. ALL WELD ELECTRODES OR WIRE SHALL AT A MINIMUM CONFORM TO E70 ELECTRODES (70 KSI YIELD).
- ALL STEEL SHAPES AND PLATES SHALL BE HOT-DIPPED GALVANIZED ACCORDING TO ASTM A123. ALL STEEL NUTS AND BOLTS AND ASSOCIATED HARDWARE SHALL BE HOT-DIPPED ACCORDING TO ASTM A153.
- WIND TESTING OF PINE TREE BRANCHES HAS BEEN COMPLETED BY THE SUPPLIER OF THE BRANCHES, LARSON. LARSON HAS VERIFIED THE STRENGTH OF THE BRANCHES THROUGH FULL SCALE WIND TESTING. THE WIND AREA USED IN THE CALCULATIONS IS BASED ON THE WIND TEST DATA. THE CALCULATION ACCOUNT FOR PINE TREE BRANCHES ATTACHED AT THE TOP OF THE MONOPOLE. ISE INC. HAS REVIEWED AND APPROVED THE WIND TEST METHODS.
- THE MAIN MONOPOLE STRUCTURE SHALL BE FABRICATED BY A JURISDICTION CERTIFIED FABRICATOR OF CONVENTIONAL STEEL STRUCTURES.
- SPECIAL INSPECTION SHALL BE PERFORMED ACCORDING TO SECTION 1704 OF THE 2018 IBC REFER TO TABLE "SUMMARY OF SPECIAL INSPECTION" ON THIS SHEET.
- IT IS THE CONTRACTORS SOLE RESPONSIBILITY TO NOTIFY THE SPECIAL INSPECTOR OR INSPECTION AGENCY (OR THE INSPECTING GEOTECHNICAL ENGINEER) AT LEAST ONE WORKING DAY PRIOR TO PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. PER THE 2018 IBC ANY WORK THAT REQUIRES SPECIAL INSPECTION THAT IS INSTALLED OR COVERED WITHOUT THE APPROVAL OF THE SPECIAL INSPECTION IS SUBJECT TO REMOVAL.
- THE LIST OF SPECIAL INSPECTIONS IS IN ADDITION TO INSPECTIONS REQUIRED BY SECTION 110 OF THE 2018 IBC. SPECIAL INSPECTION IS NOT A SUBSTITUTION FOR INSPECTION BY A CITY INSPECTOR.
- THE SPECIAL INSPECTOR SHALL BE APPROVED BY THE LOCAL JURISDICTION TO PERFORM THE TYPES OF INSPECTION REQUIRED.
- CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING THE PERFORMANCE OF THE WORK UNLESS OTHERWISE SPECIFIED.
- ANY SUPPORT SERVICE PERFORMED BY THE ENGINEER OF RECORD DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES, WHICH ARE FURNISHED BY OTHERS. THESE SUPPORT SERVICES PERFORMED BY THE ENGINEER OF RECORD ARE ONLY FOR THE PURPOSE OF ASSISTING IN THE QUALITY CONTROL AND IN ACHIEVING CONFORMANCE WITH THE CONTRACT DOCUMENTS. THIS SUPPORT DOES NOT GUARANTEE THE CONTRACTORS PERFORMANCE AND SHALL NOT BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.
- THE ANTENNA MOUNT SHALL BE FABRICATED BY LARSON CAMOUFLAGE, LLC. OR AN APPROVED FABRICATOR OF CONVENTIONAL STEEL STRUCTURES.

FOUNDATION NOTES:

- THE GEOTECHNICAL ENGINEER (OR THE APPROPRIATE INSPECTOR) SHALL INSPECT THE EXCAVATION PRIOR TO PLACING REINFORCING STEEL OR FORMS. THE GEOTECHNICAL ENGINEER (OR INSPECTOR) SHALL PROVIDE A NOTICE OF INSPECTION FOR THE BUILDING INSPECTOR FOR REVIEW AND RECORDS PURPOSE.
- THE CONTRACTOR SHALL DETERMINE THE MEANS AND METHODS TO SUPPORT THE EXCAVATION DURING CONSTRUCTION. REFER TO THE GEOTECHNICAL REPORT FOR RECOMMENDATIONS.
- THE CONTRACTOR SHALL READ THE GEOTECHNICAL REPORT AND SHALL CONSULT THE GEOTECHNICAL ENGINEER AS NECESSARY PRIOR TO CONSTRUCTION.
- FOUNDATION DESIGN PER:
 2018 IBC TABLE 1806.2 MATERIAL CLASS 5
 150 PSF/FT ALLOWABLE LATERAL BEARING
- ALL FOUNDATION CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH F_c = 4000 PSI AT 28 DAYS. CONCRETE MIX SHALL BE DESIGNED BY AN APPROVED LABORATORY. CONCRETE SHALL HAVE A MAXIMUM WATER/CEMENT RATIO OF 0.45. ALL CONCRETE CONSTRUCTION SHALL BE IN ACCORDANCE WITH ACI 318. "THE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE", LATEST EDITION. CEMENT SHALL BE TYPE II, CONFORMING TO ASTM C-150. ALL AGGREGATE USED IN THE CONCRETE SHALL CONFORM TO ASTM C-33. MAXIMUM AGGREGATE SIZE TO BE 1 1/2". SLUMP 4" - 6".
- CAISSON FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 338, "STANDARD SPECIFICATIONS FOR THE CONSTRUCTION OF DRILLED PIERS", LATEST EDITION. MAT/PIER FOUNDATION INSTALLATION SHALL BE IN ACCORDANCE WITH ACI 318 LATEST EDITION. CONCRETE COLUMNS SHALL BE MADE AND TESTED. A MINIMUM OF ONE (1) SET SHALL BE TAKEN FROM CONCRETE IN FOUNDATION. EACH SET SHALL CONSIST OF FOUR (4) COLUMNS. ONE SHALL BE TESTED AT (7) DAYS, TWO SHALL BE TESTED AT TWENTY EIGHT (28) DAYS AND THE LAST COLUMN SHALL BE A HOLD. ALL COLUMNS SHALL BE TAKEN, PREPARED AND TESTED BY A TESTING LAB IN ACCORDANCE WITH ASTM STANDARDS C172, C31 AND C39.
- ALL REINFORCING STEEL SHALL CONFORM TO ASTM A615. VERTICAL BARS SHALL BE GRADE 60, AND TIES OR STIRRUPS SHALL BE A MINIMUM OF GRADE 40. THE PLACEMENT OF ALL REINFORCEMENT SHALL CONFORM TO ACI 315, "MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES", LATEST EDITION, UNLESS OTHERWISE DETAILED ON THIS SHEET.
- ESTIMATED CONCRETE VOLUME =
 PIER: 60.1 CYD
 MAT: N/A CYD
- THE FOUNDATION HAS BEEN DESIGNED TO RESIST THE FOLLOWING FACTORED LOADS:
 MOMENT = 8480.75 FT-KIPS
 SHEAR = 66.292 KIPS
 AXIAL = 53.401 KIPS
- SPECIAL INSPECTION REQUIRED PER TABLE
- "SUMMARY OF SPECIAL INSPECTION"

ERECTION NOTES:

- ALL ANTENNA COAXIAL CABLES SHALL BE RUN INSIDE THE MONOPOLE SHAFT.
- THE CONTRACTOR SHALL INSTALL THE ANTENNA AND MOUNT AS REQUIRED BY THE OWNER.
- ALL ANCHOR BOLT NUTS SHALL BE TIGHTENED TO AISC SNUG TIGHT REQUIREMENTS. THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXISTS WHEN ALL PLIES IN A JOINT ARE IN FIRM CONTACT. THIS MAY BE ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH.
- ALL GALVANIZED SURFACES THAT ARE DAMAGED BY ABRASIONS, CUTS, DRILLING OR FIELD WELDING DURING SHIPPING OR ERECTION SHALL BE TOUCHED UP WITH TWO COATS OF A COLD GALVANIZING COMPOUND MEETING THE REQUIREMENTS OF ASTM A780.
- THE ANCHOR BOLT TEMPLATES AND BASE PLATE WILL TYPICALLY HAVE AN AZIMUTH WELDED OR A NOTCH INDICATING THE CORRECT ORIENTATION OF THE ANCHOR BOLTS. THIS IS NECESSARY TO PROPERLY ORIENT THE MONOPOLE EXIT PORTS.
- SLIP JOINT IS A FRICTION CONNECTION THAT WILL TRANSFER DESIGN FORCES WHEN THE SPECIFIED OVERLAP IS ACHIEVED. ASSEMBLY CONTRACTOR SHALL BE EXPERIENCED AND FAMILIAR WITH TAPERED POLE ASSEMBLY. CONTRACTOR SHALL CONSPICUOUSLY MARK THE LOWER POLE SECTION FOR THE MAXIMUM, DESIGN, AND MINIMUM OVERLAP DISTANCES. CONTRACTOR SHALL SLIDE SECTIONS TOGETHER AND APPLY FORCES THROUGH JACKING OR END RAM TO ACHIEVE THE DESIGN OVERLAP.
- ALL SLIP SPLICES SHALL BE JACKED TO WITHIN THE SLIP SPLICE DESIGN CRITERIA AS SHOWN ON THESE DRAWINGS. IF THE DESIGN SPLICE CANNOT BE ATTAINED ISE INC. SHALL BE CONTACTED.
- ALL A36 THREADED ROD AND U-BOLTS SHALL BE TIGHTENED TO AISC SNUG REQUIREMENTS. THE SNUG TIGHT CONDITION IS DEFINED AS THE TIGHTNESS THAT EXIST WHEN ALL PLIES IN A JOINT ARE IN FIRM CONTACT. THIS MAY BE ATTAINED BY A FEW IMPACTS OF AN IMPACT WRENCH OR THE FULL EFFORT OF A MAN USING AN ORDINARY SPUD WRENCH. A36 NUTS AND BOLTS TIGHTENING DO NOT REQUIRE SPECIAL INSPECTION.
- ANTENNA MOUNT SHALL NOT BE USED AS A CLIMBING DEVICE. WORKERS SHALL ALWAYS TIE OFF TO A SPECIFIED CLIMBING POINT.

SUMMARY OF SPECIAL INSPECTIONS

NO.	DESCRIPTION OF TYPE OF INSPECTION REQUIRED, LOCATION, REMARKS, ETC	CONTINUOUS / PERIODIC
1)	FOUNDATION CONSTRUCTION:	
A.	- GEOTECHNICAL ENGINEER OF RECORD MAY SERVE AS THE SPECIAL INSPECTOR FOR THE FOUNDATION CONSTRUCTION.	
B.	- SHALL VERIFY THE DIAMETER, DEPTH AND QUALITY OF EXCAVATION PRIOR TO THE CONCRETE PLACEMENT.	PERIODIC
C.	- SHALL VERIFY THE ON SITE SOILS ARE AS DETERMINED IN THE SOILS REPORT.	PERIODIC
2)	CAST IN PLACE CONCRETE (FOUNDATION):	
A.	- REINFORCING CAGE SHALL BE INSPECTED TO ENSURE THAT THE PROPER GEOMETRY, SIZE, LENGTH, QUANTITY AND GRADE MATERIAL ARE USED.	PERIODIC
B.	- ALL CONCRETE SHALL BE AS SPECIFIED BY ACI-318, LATEST EDITION TO ENSURE THE COMPRESSIVE STRENGTH IS ATTAINED AS DESCRIBED IN THE FOUNDATION NOTES.	
C.	- CONTINUOUS INSPECTION IS REQUIRED DURING THE CONCRETE PLACEMENT.	CONTINUOUS
3)	ANCHOR BOLTS INSTALLED IN CONCRETE:	
	- PLACEMENT SHALL BE ORIENTED ON PROPER BOLT CIRCLE AS SHOWN ON THE STRUCTURAL PLANS, WITH TOP AND BOTTOM TEMPLATES INSTALLED.	PERIODIC
	- SHALL BE PLUMB.	PERIODIC
	- SHALL HAVE A MINIMUM EMBEDMENT OF 72" INTO FOUNDATION (12" MAXIMUM PROJECTION).	PERIODIC
	- SHALL BE TIGHTENED TO SNUG TIGHT CONDITION PER AISC STEEL MANUAL OF STEEL CONSTRUCTION.	PERIODIC

SITE #: US-NC-7009
 SITE NAME: MACAFFREY
 125'-0" MONOPINE
 PRODUCT INFORMATION & NOTES

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 (LATITUDE: 36° 08' 41.31" N / LONGITUDE: 79° 57' 33.48" W)

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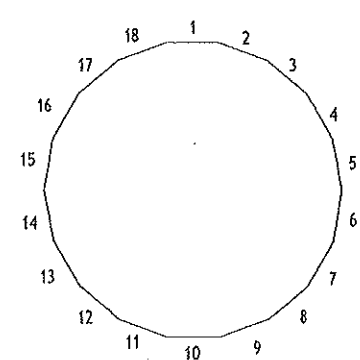


PROGRESS LOG	

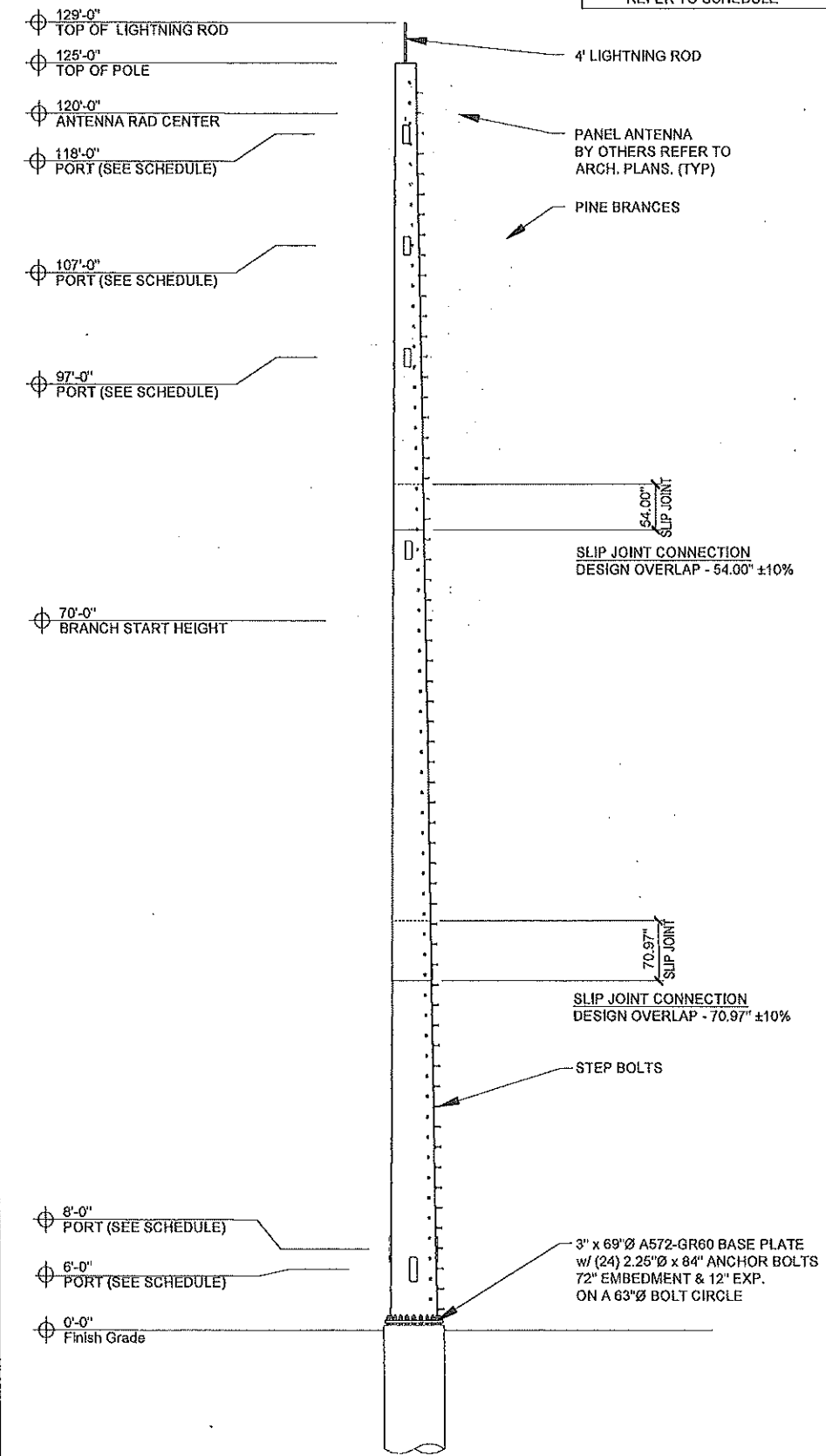
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SHEET NUMBER	PROGRESS
PF1	0

DRAWING DATE
 July 01, 2021

Dist From Top	MOUFT
0'	50
1'	50
2'	60
3'	60
4'	70
5'	70
6'	80
7'	80
8'	90
9'	90
10'	90
11'	90
12'	90
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52'	90
53'	90
54'	90
55'	90
56'	90
57'	90
58'	90
59'	90
60'	90



NOTES:
 1. PINE BRANCHES SHOWN FOR ILLUSTRATION PURPOSE ONLY
 2. HANDHOLES NOT SHOWN AT TRUE ORIENTATION. REFER TO SCHEDULE



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LARSON
 A valmont COMPANY
 LARSON JOB #: P520686

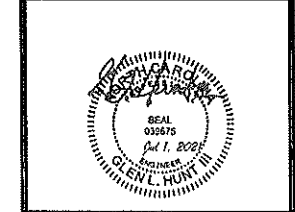
ISE Incorporated
 Structural Engineers
 P.O. BOX 50239
 Phoenix, Arizona 85074
 PHONE: 602-998-8141
 www.ise.com

ISE JOB #: 17030

SITE #: US-NC-7009
 SITE NAME: MACAFFREY
 125'-0" MONOPINE
 POLE ELEVATION

3525 EDGEFIELD RD, GREENSBORO, NC 27409
 (LATITUDE: 36° 08' 41.31" N / LONGITUDE: 79° 57' 53.48" W)

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PROGRESS LOG

ISSUED TO CLIENT	MG
SHEET NUMBER	PROGRESS
PF2	0

DRAWING DATE
 July 01, 2021

SITE #: US-NC-7009
SITE NAME: MACAFFREY
125'-0" MONOPINE
PIER FOUNDATION
3529 EDGEFIELD RD, GREENSBORO, NC 27409
(LATITUDE: 36° 08' 41.31" N / LONGITUDE: 79° 57' 33.45" W)

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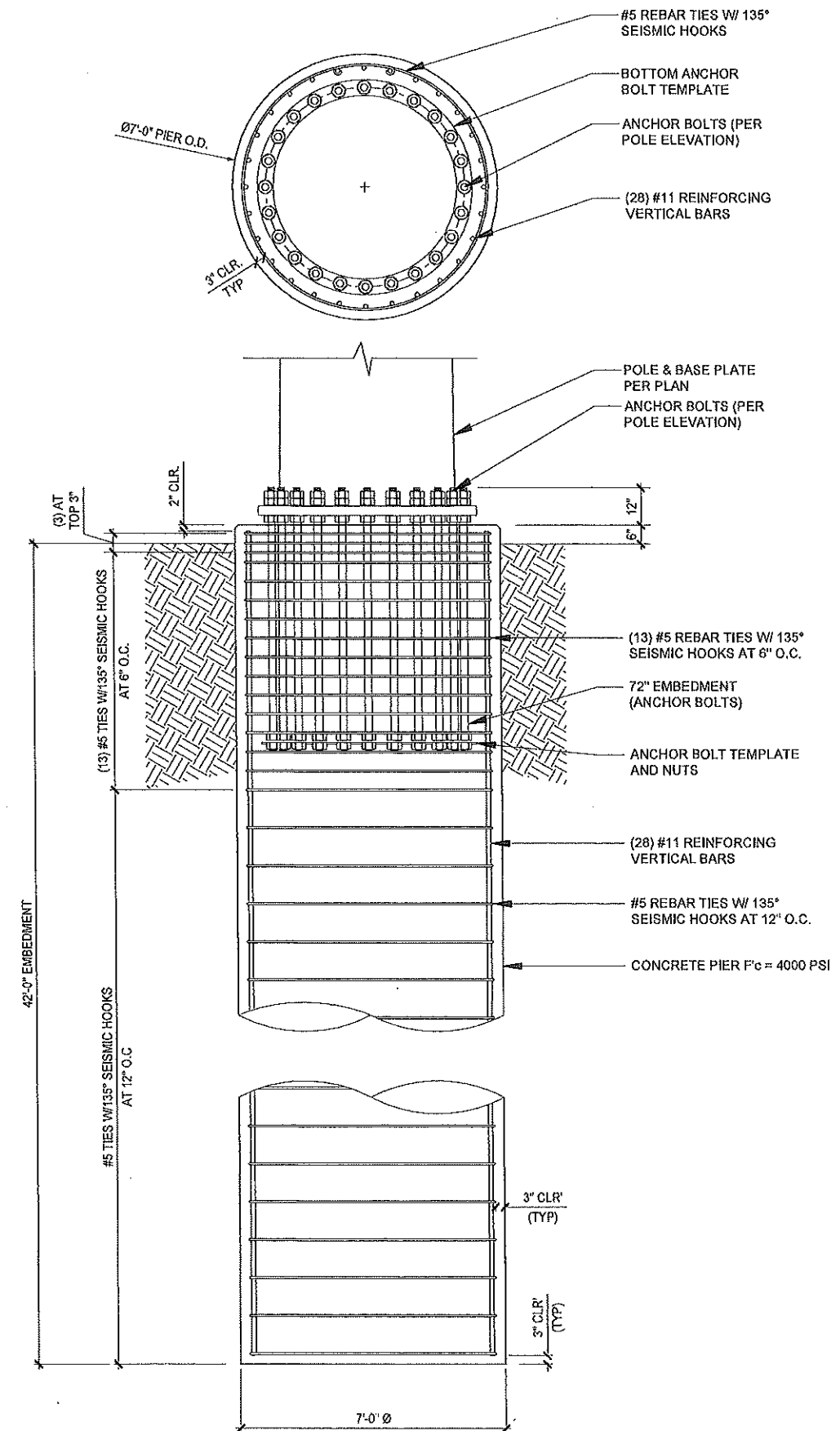


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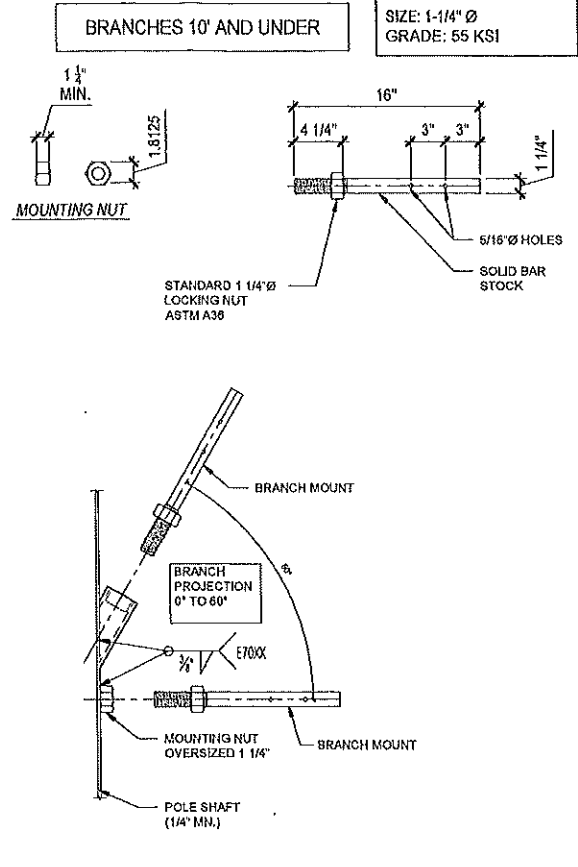
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DRAWING DATE: July 01, 2021

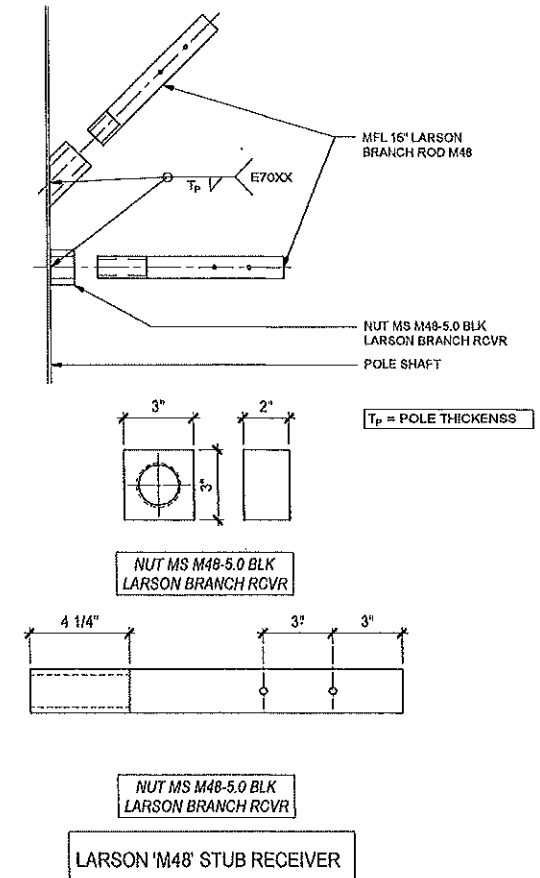


1 PIER FOUNDATION

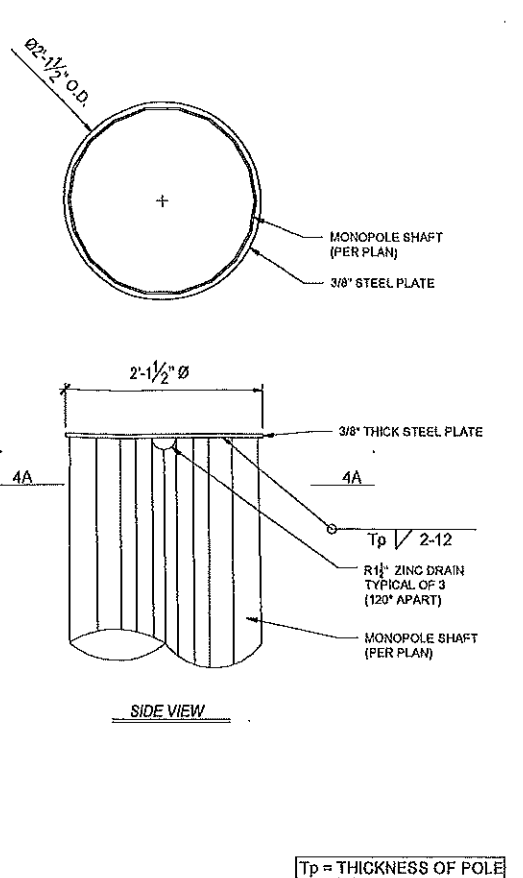
SCALE: N.T.S.



9 BRANCH RECEIVER DETAIL (10' & shorter) SCALE: N.T.S.



7 BRANCH RECEIVER DETAIL (over 10') SCALE: N.T.S.

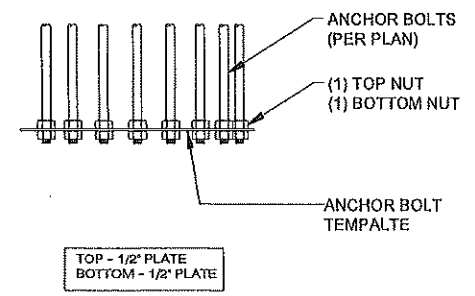


4 TOP FLANGE PLATE DETAIL SCALE: N.T.S.

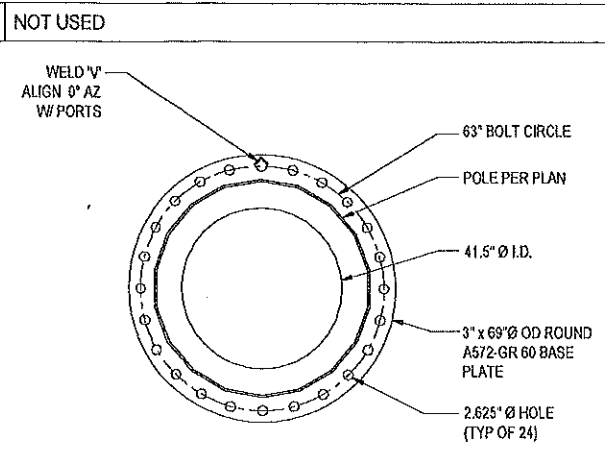
COAX HAND/ACCESS HOLE SCHEDULE

ELEV (AFG)	QTY	W (IN)	H (IN)	AZIMUTH	D1 (IN)	D2 (IN)	TI (IN)
118'-0"	3	8	22	0°, 120°, 240°	1 1/2	3 1/2	1/2"
107'-0"	3	8	22	0°, 120°, 240°	1 1/2	3 1/2	1/2"
97'-0"	3	8	22	0°, 120°, 240°	1 1/2	3 1/2	1/2"
8'-0"	2	10	30	90°, 270°	2 1/2	4 1/2	1"
6'-0"	2	10	30	0°, 180°	2 1/2	4 1/2	1"

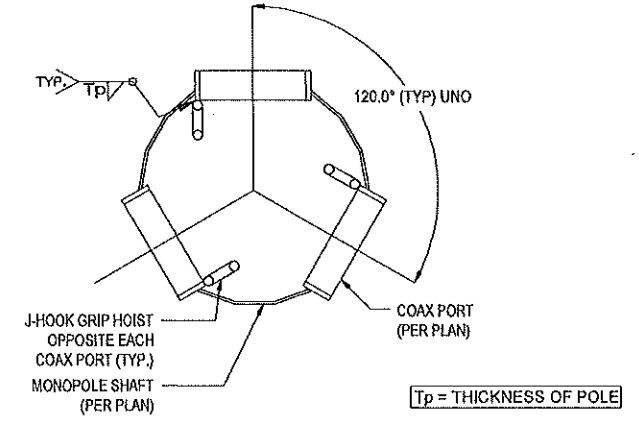
1 COAX HAND HOLE DETAILS SCALE: N.T.S.



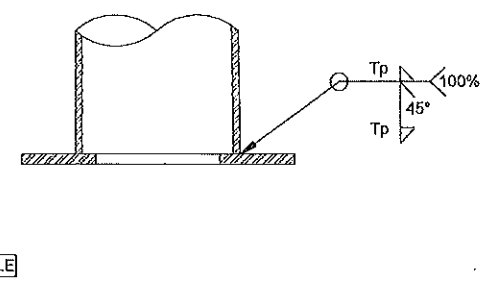
8 ANCHOR BOLT TEMPLATE (TOP AND BOTTOM) SCALE: N.T.S.



6 BASE PLATE DETAIL SCALE: N.T.S.



2 J-HOOK INSIDE POLE AT COAX PORTS SCALE: N.T.S.



3 POLE / BASE PLATE CONNECTION SCALE: N.T.S.

verizon

LARSON

A valmont COMPANY

LARSON JOB #: P520686

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Structural Engineers

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ISE JOB #: 17030

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125'-0" MONOPINE
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PROGRESS LOG

DATE	ISSUED TO CLIENT	BY
07/01/21	ISSUED TO CLIENT	JAG

SHEET NUMBER: PF3
PROGRESS: 0

DRAWING DATE: July 01, 2021